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Snippets from Infiltration: where Approximate Integral Analysis is Exact.

Hydrology of 1D Unsaturated Flow

in Darcy-Buckingham-Richards approach. Nonlinear diffusion-convection equations

$$\frac{\partial \theta}{\partial t} = \frac{\partial}{\partial z} [D(\theta) \frac{\partial \theta}{\partial z}] - K'(\theta) \frac{\partial \theta}{\partial z}$$

$$D(\theta) = K(\theta)\Psi'(\theta)$$

 θ = volumetric H₂O conc.

 Ψ = capillary suction head.

Some strongly influential papers of the 70s:

- J.-Y. Parlange, "Theory of water movement in soils, 8, One-dimensional infiltration with constant flux at the surface, Soil Sci 114, 1-4 (1972).
- -Intro of approx analytical method based on mass conservation in integral form
- W. Brutsaert, "Universal constants for scaling the exponential soil water diffusivity?", Water Resour Res 15, 481--483 (1979).
- -Benefits of simplifying flow problems by sorptivity-based scaling

Realistic Integrable Model

$$D(\theta) = \frac{a}{(b-\theta)^2} \quad ; \quad K(\theta) = \frac{\lambda/2}{b-\theta} + \gamma(b-\theta) + \beta$$

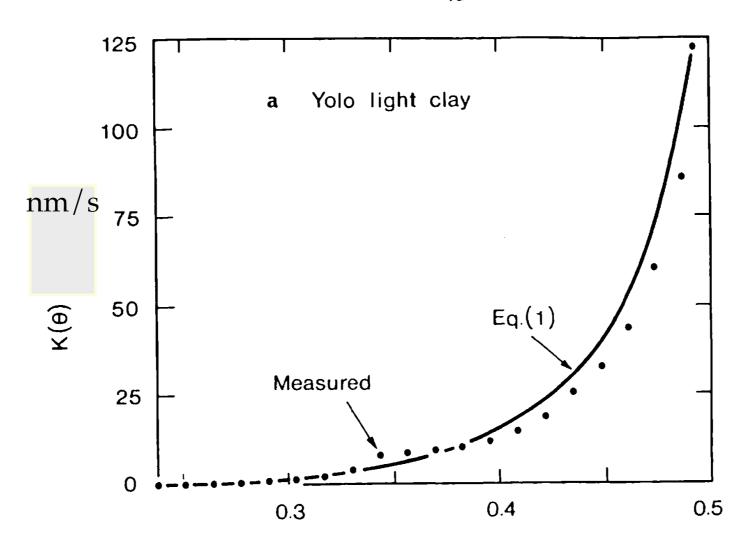
Solution with constant-flux boundary cond:

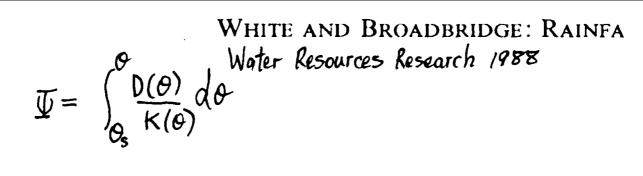
Sander, Parlange, Kuehnel, Hogarth, Lockington and O'Kane, J. Hydrol 97: 341-346 (1988).

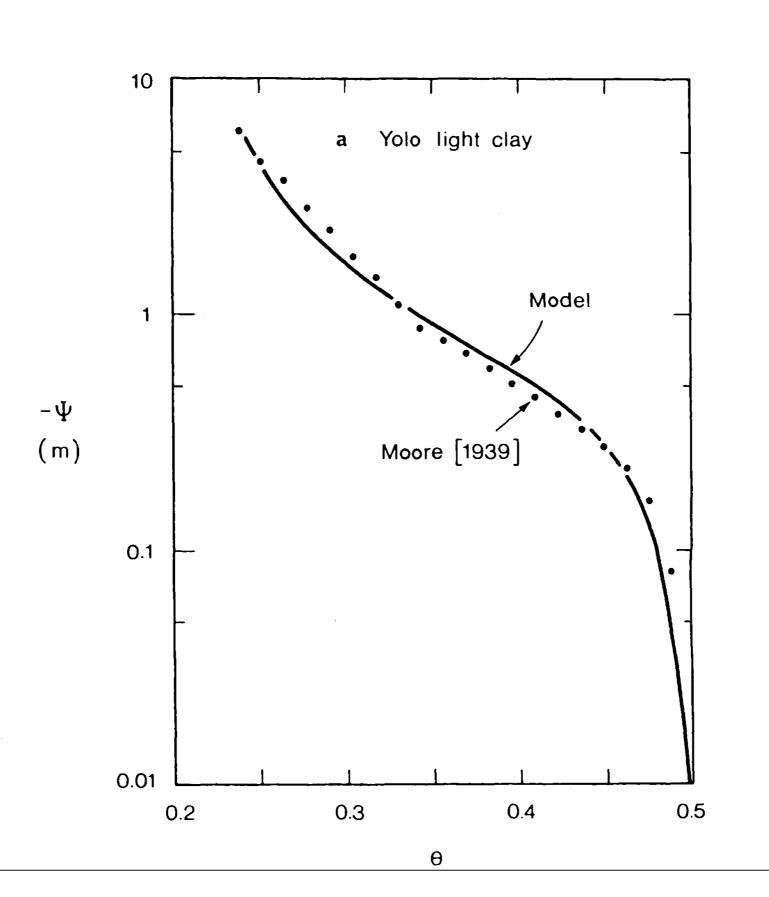
Broadbridge and White, WRR 24(1), 145-154 (1988).

Exact ponding time: B&W, WRR 23(12), 2302-10 (1987).

WHITE AND BROADBRIDGE: I Water Resources Research 1988







$$D(\theta) = \frac{a}{(b-\theta)^2} \quad ; \quad K(\theta) = \frac{\lambda/2}{b-\theta} + \gamma(b-\theta) + \beta$$

$$\ell_s = \frac{1}{K_s - K_n} \int_{\theta_n}^{\theta_s} D(\theta) d\theta = \frac{1}{K_s - K_n} \int_{\Psi_n}^{0} K d\Psi$$

$$t_s = \frac{\ell_s(\theta_s - \theta_n)}{K_s - K_n}$$

$$\bar{D} = \frac{1}{\theta_s - \theta_n} \int_{\theta_n}^{\theta_s} D(\theta) \ d\theta = \frac{h(C)}{C(C-1)} \frac{S_0^2}{(\theta_s - \theta_n)^2}$$

$$1 < C < \infty \; ; \; \frac{1}{2} < \frac{h(C)}{C(C-1)} < \frac{\pi}{4}$$

 $1/\sqrt{h(C)} = \text{sol'} n$ of transcendent eq for location of melting front in classical Stefan prob.

Results will be shown for

$$D_*(\Theta) = \frac{C(C-1)}{(C-\Theta)^2}$$

$$K_*(\Theta) = \zeta \frac{C(C-1)}{C-\Theta} + (\zeta-1)(C-\Theta)$$

$$\zeta = 1 + \frac{(\theta_s - \theta_n)\gamma}{K_s - K_n}.$$

$$\frac{\partial \Theta}{\partial t_*} = \frac{\partial}{\partial z_*} \left(D_*(\Theta) \frac{\partial \Theta}{\partial z_*} - K_*(\Theta) \right)$$

$$\Theta(z_*,0) = 0 \qquad z_* \in (0,\infty)$$

$$\Theta(z_*,t_*) \to 0$$
 as $z_* \to \infty$

+ flux boundary cond

$$K_*(\Theta) - D_*(\Theta) \frac{\partial \Theta}{\partial z_*} = R_* \; ; \; z_* = 0$$

or concentration boundary cond

$$\Theta(0, t_*) = 1 \qquad t_* \in (0, \infty)$$





 $\zeta = C$ implies model of Broadbridge & White 1988 with $K_*'(0) = 0$

$$\zeta = 1. \implies K'(\theta)/D(\theta)const. \implies K = e^{\alpha \Psi}$$

$$(C,\zeta) \to (1,1) \implies D_*(\Theta) \to \delta(\Theta-1), K_* \to H(\Theta-1)$$

$$(C,\zeta) \to (\infty,\infty) \implies D_*(\Theta) \to 1, K_* \to \Theta^2$$

$$(C,\zeta) \to (\infty,0) \implies D_*(\Theta) \to 1, \quad K'_*(\Theta) \to 1$$

$$(C,\zeta) \to (1,0) \implies D_*(\Theta) \to \delta(\Theta-1), \quad K'_*(\Theta) \to 1$$

leads to predictions of Green-Ampt model

"Quasi-analytic" integral approximation method: integrate by parts through Richards' eq.,

$$-\frac{\partial}{\partial t} \int_{\theta_n}^{\theta} z(\bar{\theta}, t) d\bar{\theta} = D \frac{\partial \theta}{\partial z} - K + K_n$$

$$\frac{1}{2} \frac{\partial}{\partial t} \int_{\theta_n}^{\theta_s} z^2(\theta, t) d\theta = \int_{\theta_n}^{\theta_s} D(\theta) d\theta - \int_{\theta_n}^{\theta_s} (K - K_n) \frac{\partial z}{\partial \theta} d\theta.$$

then make safe approximations within the integrand.

Time to ponding under constant irrigation rate:

$$\Theta(0, t_p^*) = 1$$

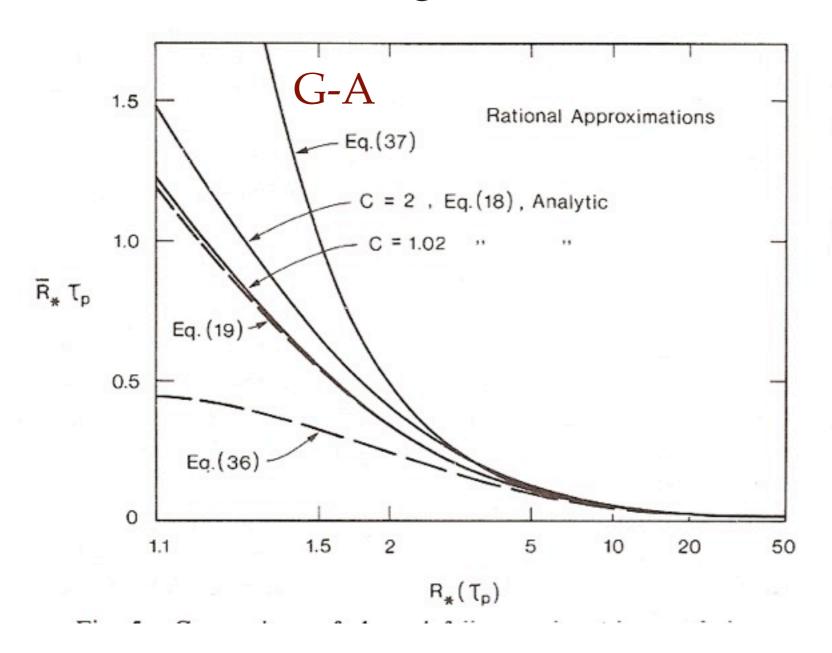
Approximate analysis Parlange and Smith 1976:

$$\bar{R}_* t_p^* = \ln\left(\frac{R_*(t_p^*)}{R_*(t_p^*) - 1}\right)$$
 ; $R_* = \frac{R - K_n}{K_s - K_n}$

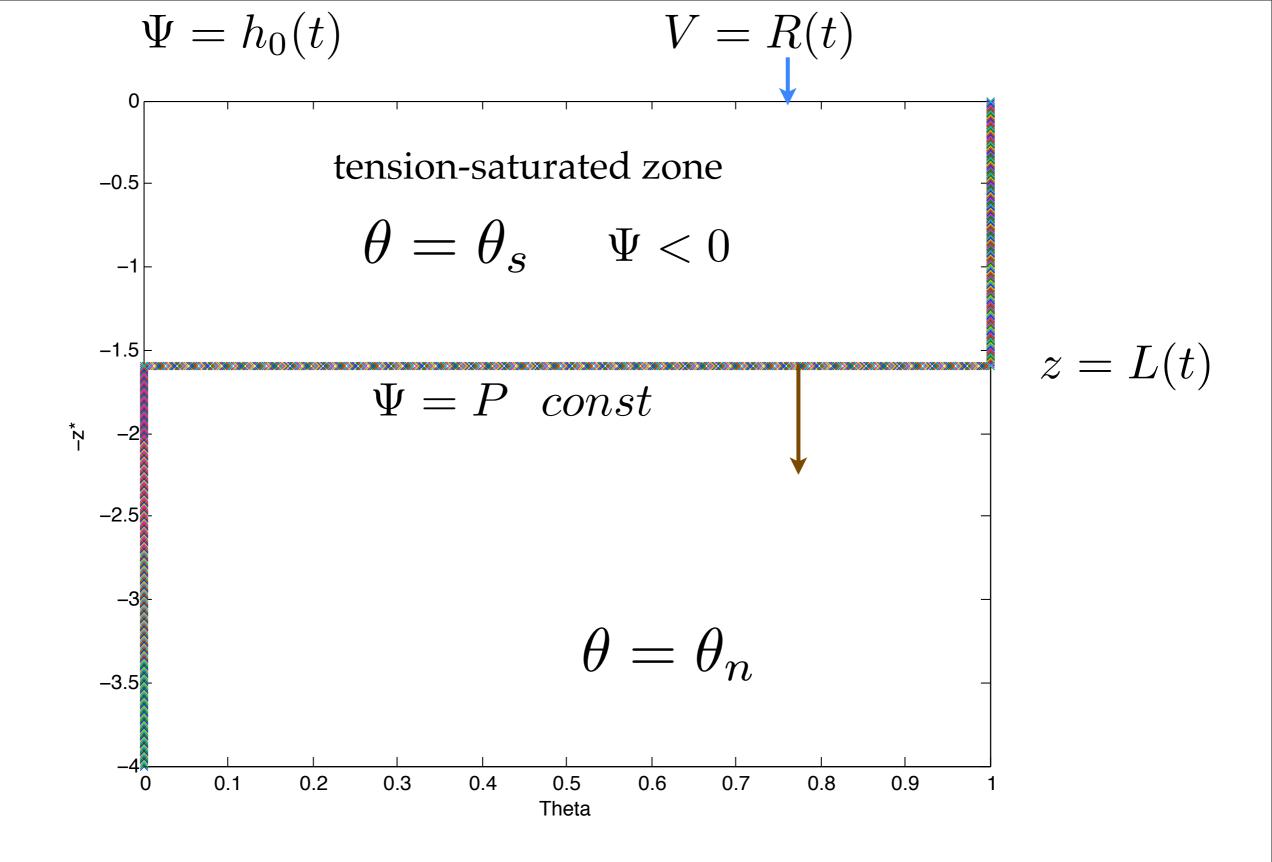
Exactly solvable model BW 1987:

$$(C,\zeta) \to 1 \; ; \quad t_p^* \to \frac{1}{R_*} \ln\left(\frac{R_*}{R_*-1}\right)$$

Broadbridge & White 1987 $C = \zeta$

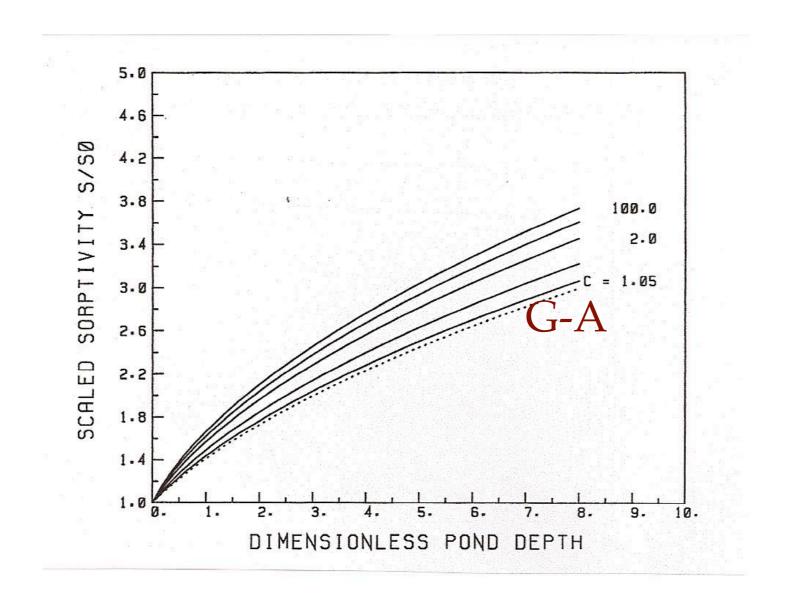


g=0 implies t_p =0.5 S_0^2/R^2 (Eq. 36 graphed above). (Parlange & Smith 1976 Eq. 19 graphed above)



Green & Ampt 1911

Effect of pond depth on sorptivity: Broadbridge WRR 1990



Green-Ampt model:

$$S/S_0 = (1 + \Psi_0/|P|)^{1/2}$$

Parlange, Haverkamp & Touma 1985:

$$\mu = \frac{\int_{\theta_n}^{\theta_s} (\theta_s - \theta) Dd\theta}{S_0^2 + 2K_s \psi_0(\theta_s - \theta_i)}$$

$$S/S_0 = (1 + [1 + \mu]\Psi_0/|P|)^{1/2} \pm 1\%$$

For all values $C \in [1, \infty]$, error is less than 1%.

Exact infiltration coeff's: Triadis & Broadbridge 2011

$$i(t) = \int_0^\infty (\theta - \theta_n) dz + K_n t = i^-(t) + K_n t$$

$$i^{-}(t) = S_0 t^{1/2} + S_1 t + \dots + S_j t^{(1+j)/2} + \dots$$

Problem: Green-Ampt model predicts

$$S_1/K_s = 2/3$$

but field observations for soils show

$$0.3 < S_1/K_s < 0.4$$

e.g. Talsma 1969: $S_1/K_s = 1/2.8 = 0.357...$

$$\begin{split} \frac{S_1(C,\zeta)}{K_s - K_n} &= \frac{1}{2} \\ + (2\zeta - 1)\sqrt{C(C - 1)} \ \frac{4\sqrt{(C - 1)/C} \ \Psi(1, 1/2, \gamma_0^2/4)}{4\Psi(1, 1/2, \gamma_0^2/4) - \gamma_0^2\Psi(2, 3/2, \gamma_0^2/4)} \end{split}$$

Where Ψ is Kummer-U generalized hypergeometric function and $\gamma_0^2=1/h(C)$.

At least S₁,...,S₇ agree exactly with

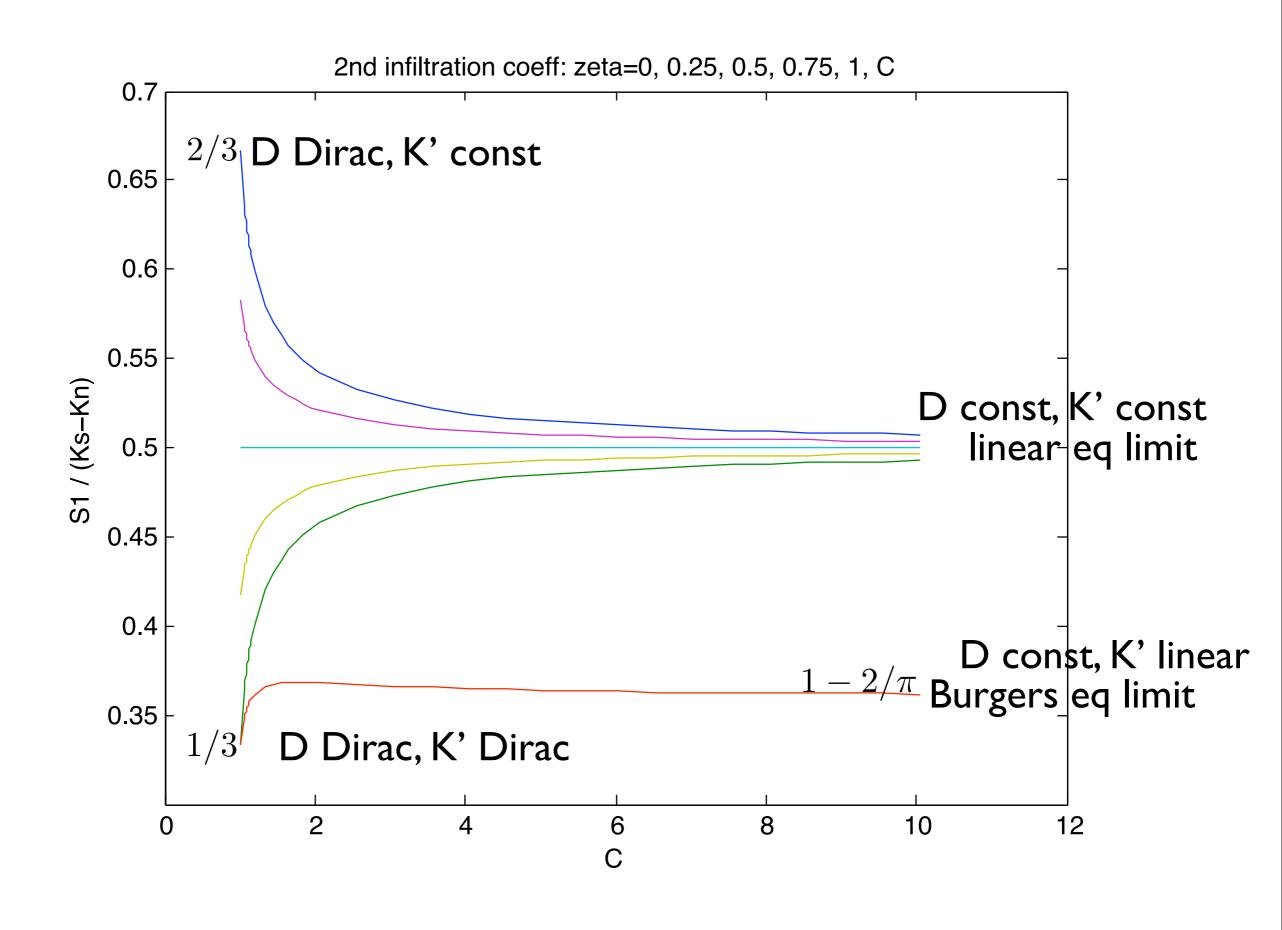
$$t_* = \frac{1}{\zeta - 1} [log\left(\frac{\zeta - 1 + e^{\zeta i_*}}{\zeta}\right) - i_*] \; ; \; C \to 1$$

$$t_* = e^{-i_*} + i_* - 1 \; ; \; (C, \zeta) \to 1$$

same as Parlange et al 1982 if their alpha=our zeta.

Talsma 1969: $S_1/K_s = 1/2.8 = 0.357...$ exactly true when $(C, \zeta) = (1.05, 1.04)$.

Water content profile is close to step function but S₁ is quite different from G-A prediction.



Main conclusions:

1. After judicious rescaling, approximate integral analysis leads to remarkably simple expressions for measurable quantities from infiltration experiments.

1a: time to ponding

1b: effect of pond depth on sorptivity

1c: second and higher infiltration coefficients

2. These expressions are satisfied exactly by some model Richards equations with reasonable nonlinear hydraulic functions.

